



HRVATSKA KOMORA INŽENJERA GRAĐEVINARSTVA

15. Dani Hrvatske komore inženjera građevinarstva

Opatija, 2021.

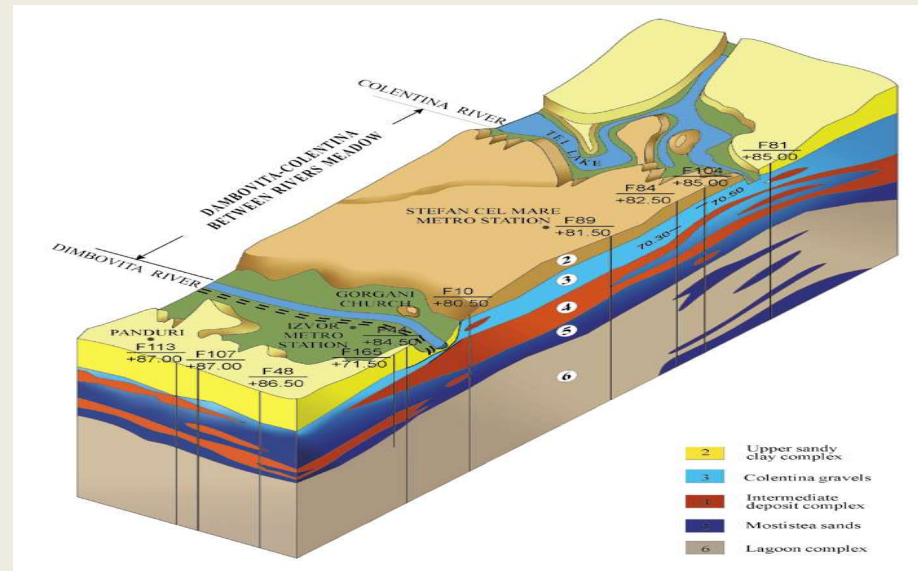
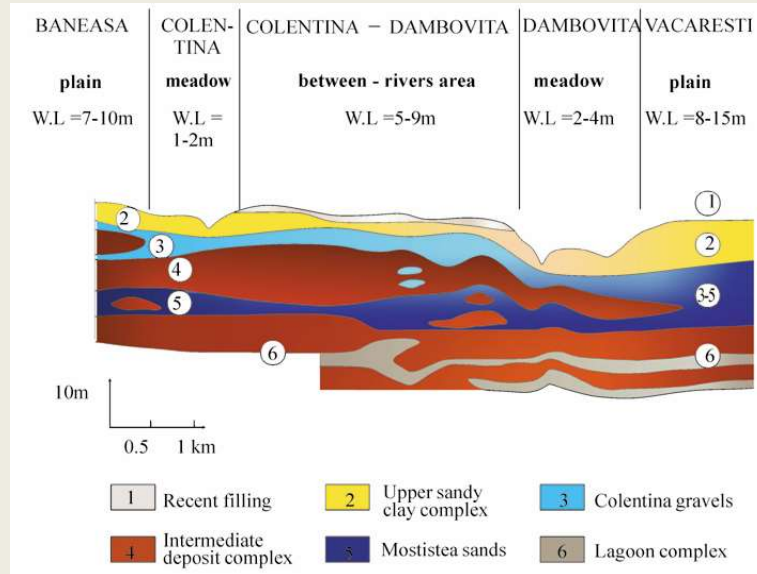
# Ground Conditions in Bucharest Metro

**Ovidiu Arghiroiu**

Ovidiu Arghiroiu, PhD, Lecturer, University of Oradea, Romania.



# GEOTECHNICAL & HYDROGEOLOGICAL CONDITIONS



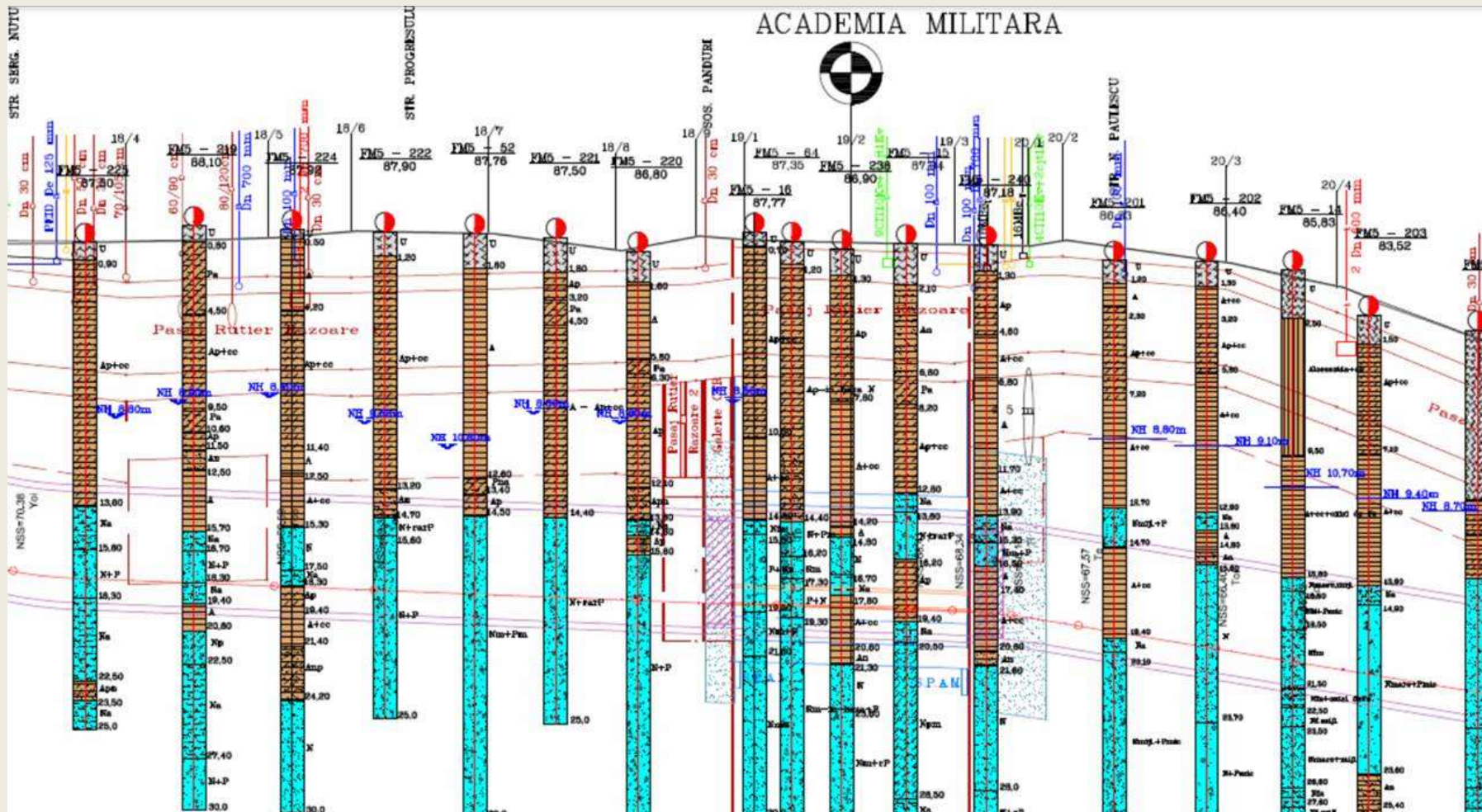
- Type 1 layer – non-uniform and heterogeneous fills, in different consolidation stages.
- Type 2 (upper clay) layer – upper clay complex, encompassing silty clays and sandy clays, with thickness ranging from 2 to 10 m.
- Type 3 (upper sand) layer – sand and gravel complex (Colentina), composed by medium to large sands and small gravels, with thickness ranging from 5 to 9 m.
- Type 4 (lower clay) layer – intermediate clay complex, composed by clays with limestone concretions, and silty clays.
- Type 5 (lower sand) layer – sand complex (Mostistea), composed by medium to fine sands and dusty sands, with great thickness, normally deeper than borehole depths





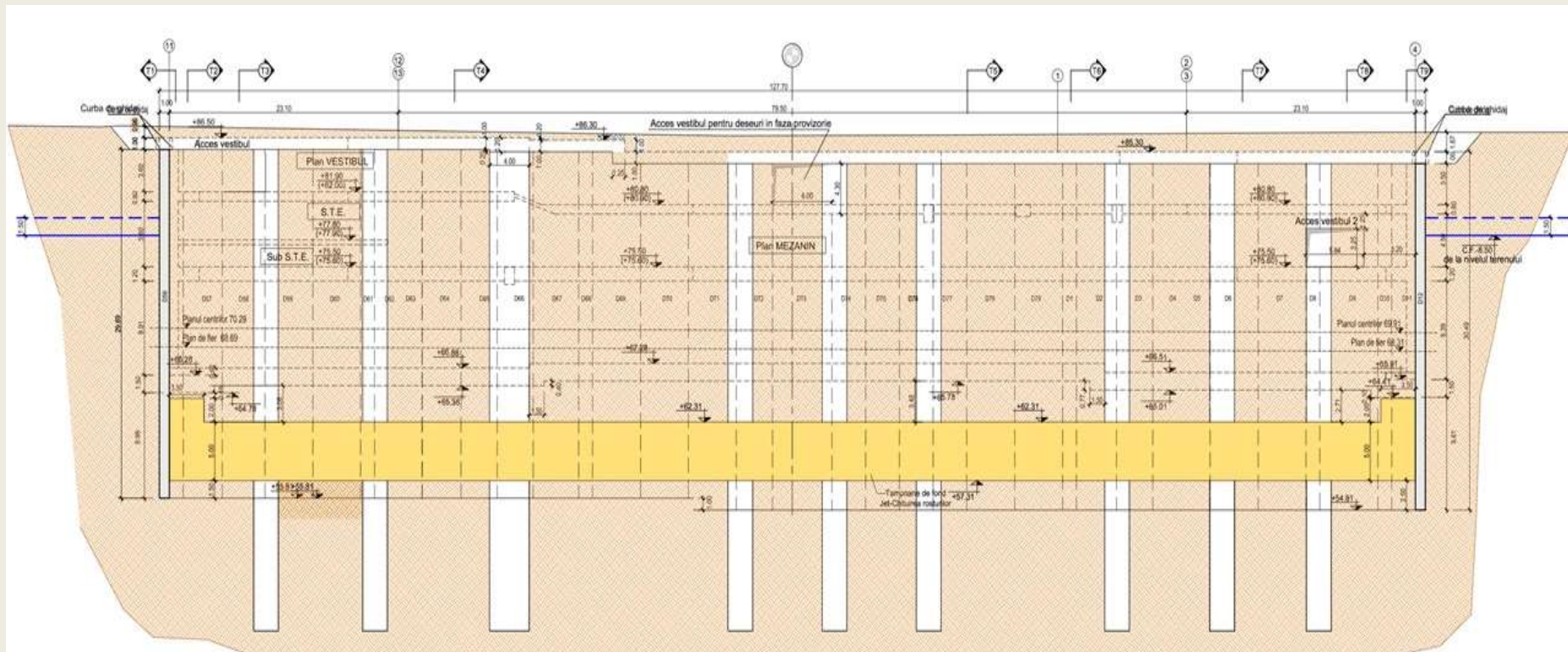
# GEOTECHNICAL and HYDROGEOLOGICAL CONDITIONS

## ACADEMIA MILITARA STATION (Line 5)



# GEOTECHNICAL and HYDROGEOLOGICAL CONDITIONS

## ACADEMIA MILITARA STATION (Line 5) BOTTOM PAD OF THE JET-GROUTING TREATMENT

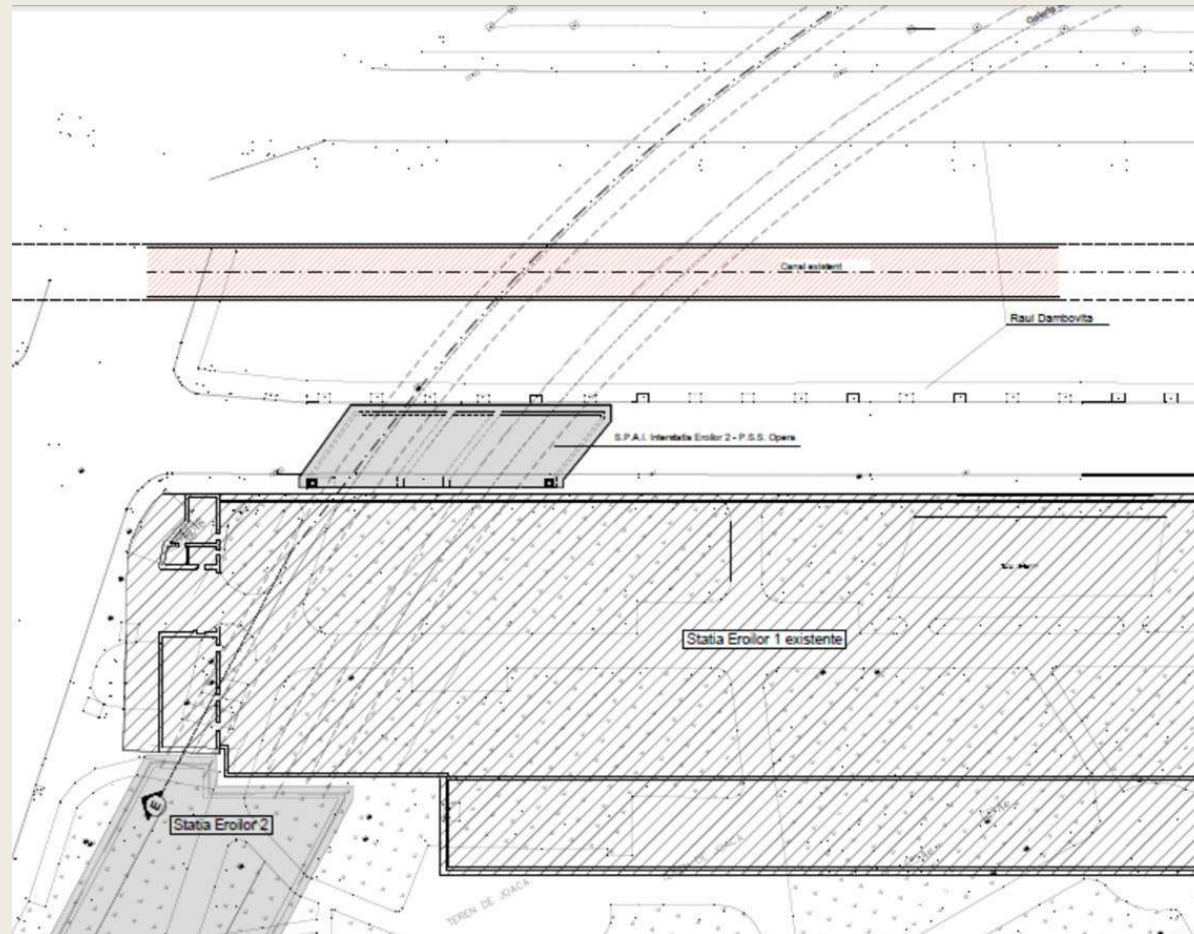






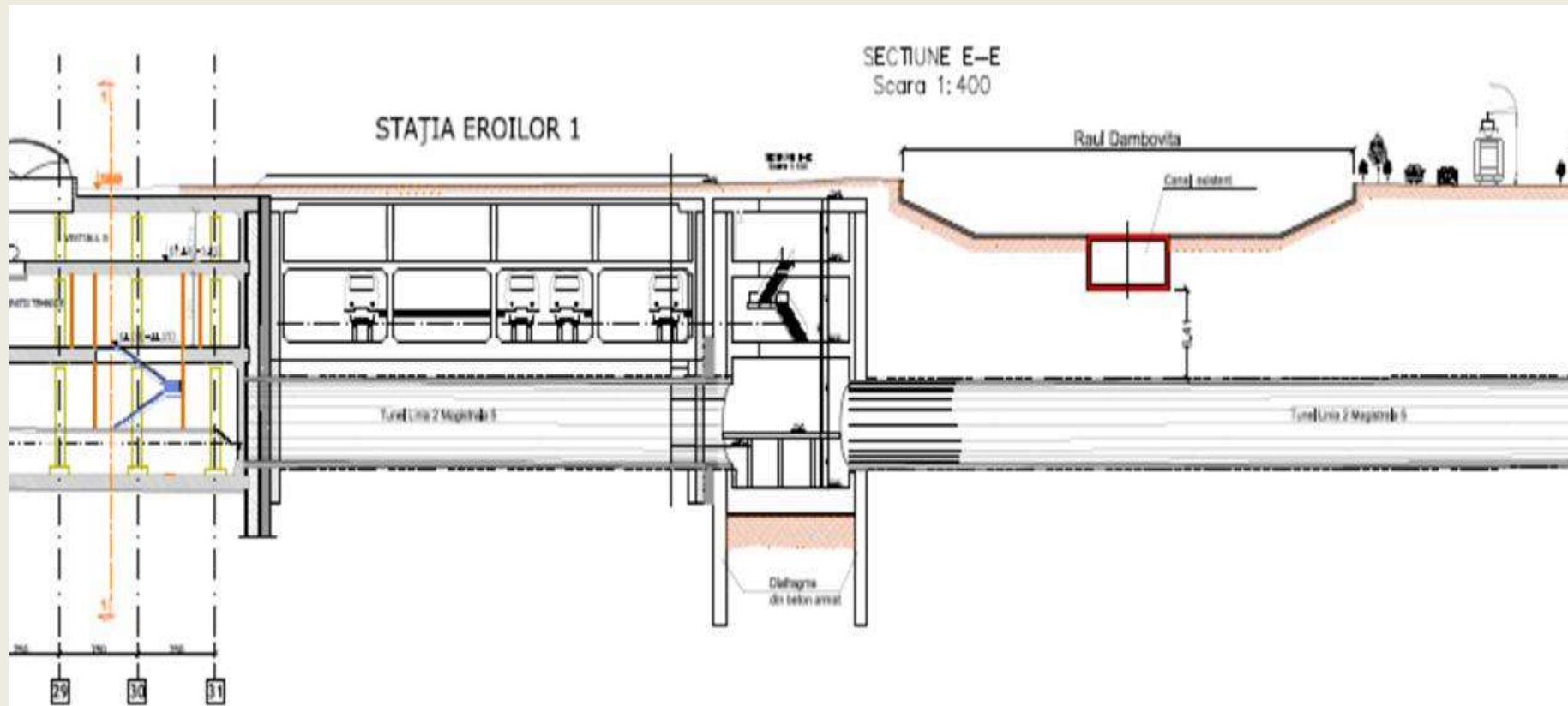
# GEOTECHNICAL and HYDROGEOLOGICAL CONDITIONS

## EROILOR 2 STATION (Line 5) – PLAN VIEW



# GEOTECHNICAL and HYDROGEOLOGICAL CONDITIONS

## EROILOR 2 STATION (Line 5) – CROSS SECTION





# SEISMIC CONDITIONS

## SEISMIC ACTION - Evolution of regulations

The first regulation dates from December 1941, after the severe Vrancea earthquake of November 10, 1940. Based on the Italian norm of 1938, this regulation considered an equal basic seismic force with 5% of the resultant of gravitational forces, evenly distributed on the floors of the building.

„The Conditional Norm for the Design of Civil and Industrial Constructions from Seismic Regions ” - P13-63, entered into force on July 18, 1963, being drawn up in accordance with "Basic Rules for Designing Buildings in Seismic Regions" written in CAER (Mutual Economic Assistance Council created by USSR). At that time, however, they did not exist recordings of seismic movements in locations located in Romania.

On December 31, 1970, the revised edition of normative P13-63 was approved, with the name “Normative for the Design of Civil and Industrial Constructions in seismic regions” - P13-70 (reduction of conventional seismic force with about 20% for reinforced concrete structures).

The effects of the earthquake of March 4, 1977, the conclusions obtained from the observations „in situ”, as well as the registration of the ground acceleration at the INCERC-Bucharest seismic station in during this major Vrancea earthquake, determined the successive elaboration of two new ones regulations: “Norm for the Anti-seismic Design of Social-cultural Housing & Agro-zootechnical and Industrial Constructions” - P100-78 (with experimental application) and the Norm P100-81.

After about 10 years, based on specialized research conducted in the country and on the plan as well as movements recorded in national seismic networks in earthquakes strong magnitude from August 30, 1986 and May 31, 1990, the Regulations P100-91 and P100-92. These seismic design standards have been developed in a modern way, the effects associated with the location of the site and the terrain conditions being included in the map seismic macrozoning and in the zoning map according to the corner period  $T_c$ .



# SEISMIC CONDITIONS

## SEISMIC ACTION - Evolution of regulations

**Equivalent Lateral Force Method** (P100/2004 - it applies only to structures that can be reduced to two-way plane systems orthogonal and whose total response is not significantly influenced by the upper modes of vibration) - the equivalent seismic forces are obtained by the independent treatment of each its own way of vibration  $k$ , characterized by its own period of vibration  $T_k$ , its own vector of vibration  $s_k$  and equivalent modal mass  $m_k$ . Basic seismic force corresponding to the vibration mode  $r$  was established as a fraction of the resultant gravitational load issues:

$$S_r = c_r G, \text{ where } c_r - \text{global seismic coefficient}$$

$$c_r = \alpha k_s \beta_r \psi \xi_r$$

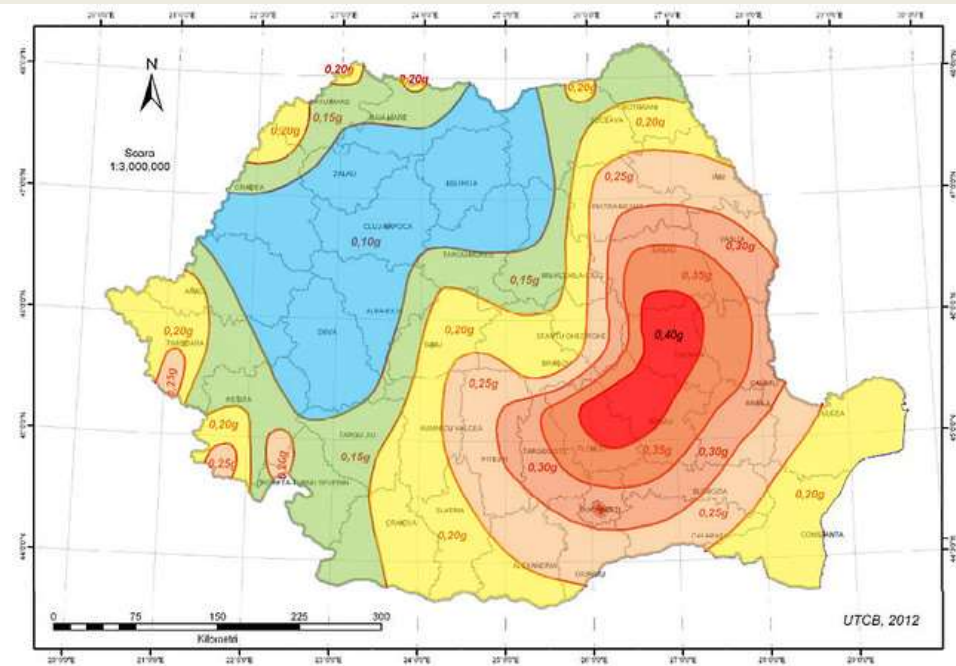
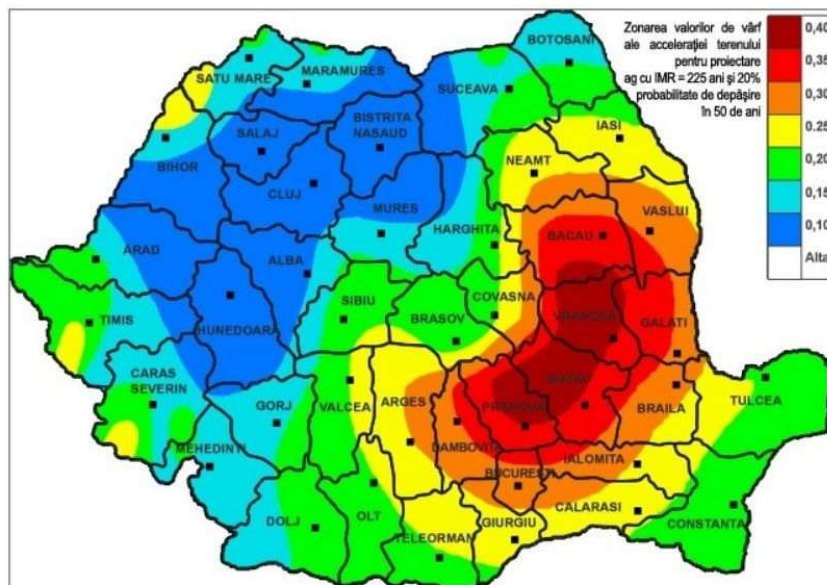
- $\alpha = \gamma_I$  - importance coefficient;
- $k_s = \frac{a_{max}}{g} = \frac{a_g}{g}$  - coefficient depending on the seismic calculation area, which depends on the maximum value of the  $a_g$  terrain acceleration, established in the new code for an average recurrence interval of 100 years;
- $\beta_r = \frac{T_r}{a_g}$  - the coefficient of dynamic amplification in its own way  $r$ , expressed by the ordinate (corresponding to the proper period  $T_r$ ) from the normalized elastic response spectrum of absolute accelerations to maximum ground acceleration;
- $\psi$  - the coefficient of reduction of the effects of the seismic action, taking account of the postelastic deformation capacities of the structures;
- $\xi_r$  - the modal equivalence coefficient.



# SEISMIC CONDITIONS

## SEISMIC ACTION – today regulations

**Response Spectrum Method** (P100/2013) - the territory of Romania it is divided into areas of seismic hazard. The level of seismic hazard in each area is considered, simplified, to be constant. The seismic hazard for design is described by the **ground peak acceleration value horizontal seismic**,  $a_g$  determined for a mean recurrence interval (IMR = 225 years, with probability of exceeding 20% in 50 years) reference value, hereinafter referred to as “design ground acceleration”. Seismic motion at a point on the surface of the ground is represented by elastic response spectra for absolute accelerations.

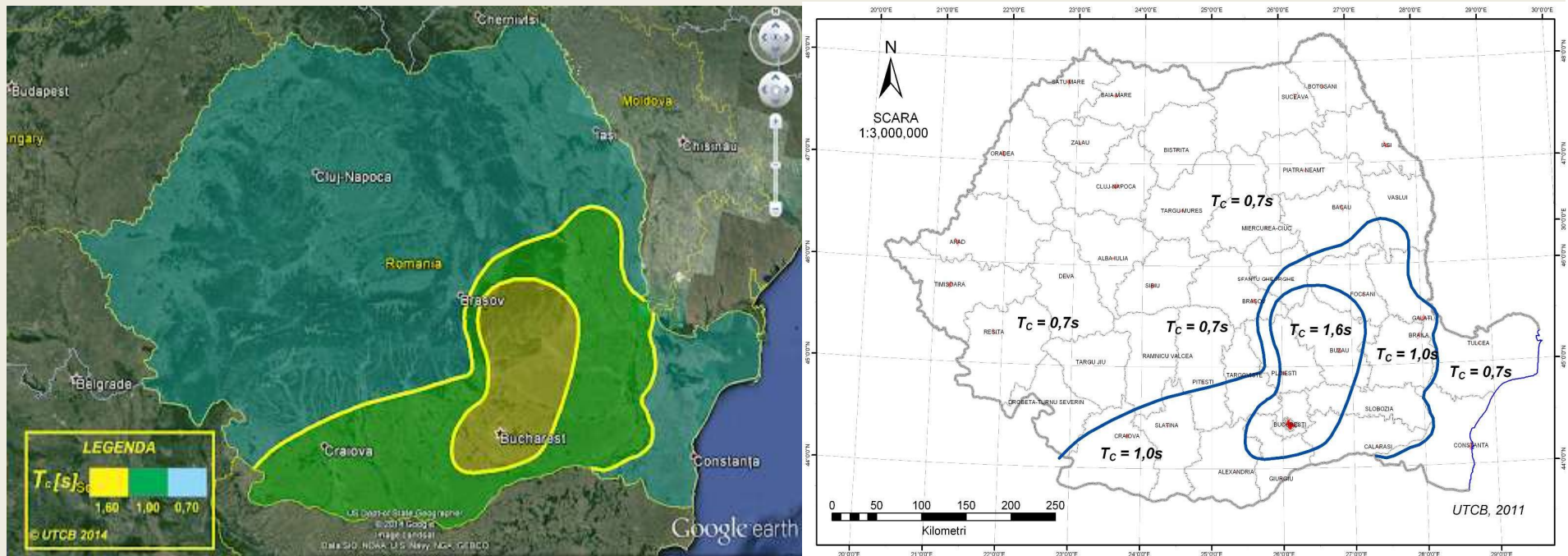




# SEISMIC CONDITIONS

## SEISMIC ACTION – today regulations

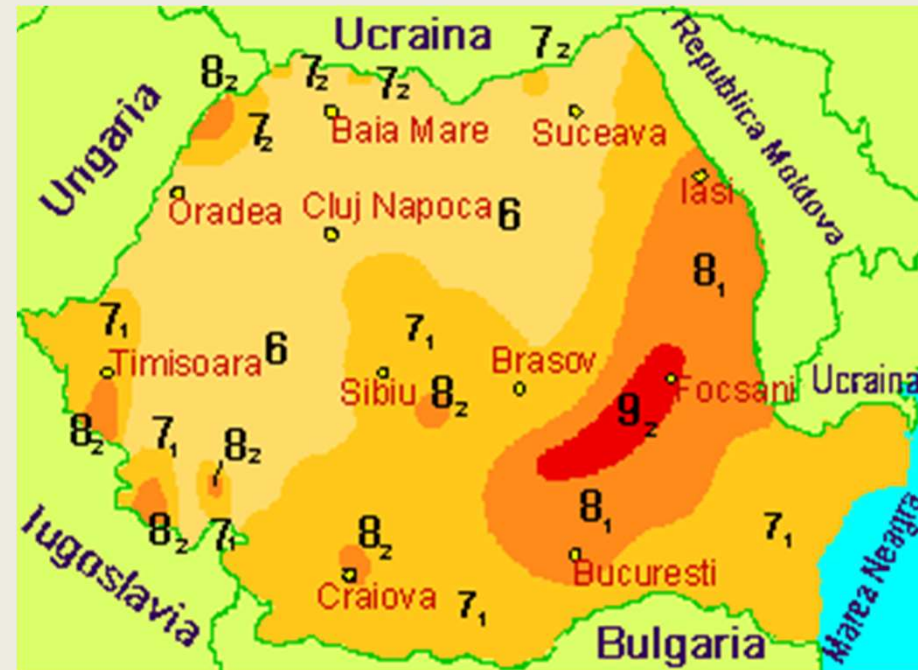
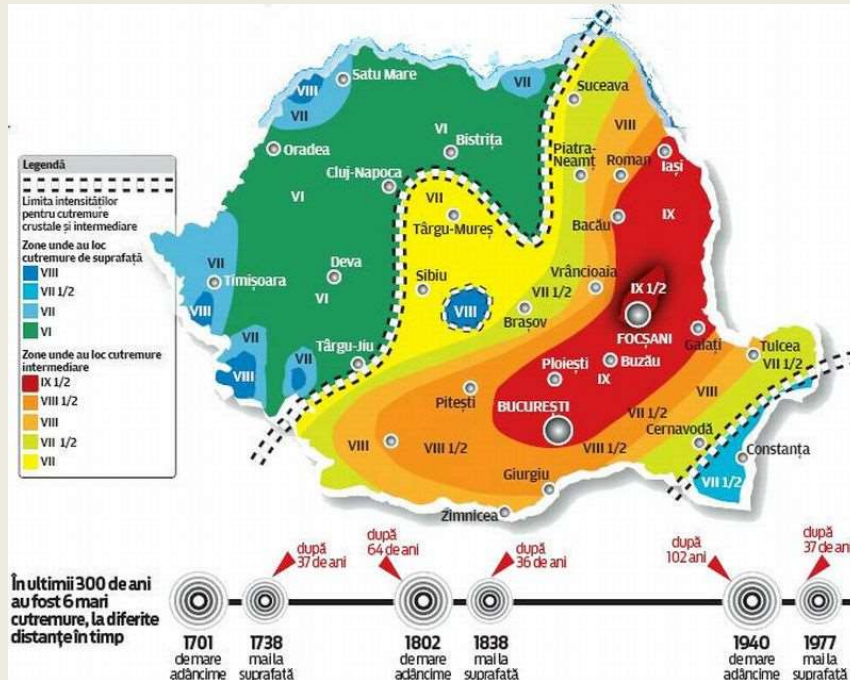
**Response Spectrum Method** (P100/2013) - the control (corner) period  $T_c$  of the response spectrum represents the boundary between the area (level) of maximum values in the spectrum of absolute accelerations and the area (level) of maximum values in the relative velocity spectrum. In fact, the  $T_c$  corner period of the design spectrum is the time required to achieve a complete oscillation of seismic waves specific to an area and characterizes its seismic conditions (expressed in seconds). Local field conditions are described simplified by the values of the control period (corner)  $T_c$  of the response spectrum for the area of the considered location. These values synthetically characterizes the frequency composition of seismic movements.





# SEISMIC CONDITIONS

## SEISMIC ACTION – zoning maps



In Vrancea there are two types of earthquakes: crustal ones - shallow (less than 60 km depth), relatively low energy and quite small magnitudes – max. 5.2 degrees on the Richter scale and subcrustal, intermediate, deep (between 60 and 220 km depth) - the latter are very dangerous, reach large magnitudes (about 7.8 ÷ 8.0 degrees Richter) and are felt over very large areas, causing large and very large damage.

In the other seismic areas of the country there are shallow earthquakes (between 5-35 km), with smaller magnitudes than the earthquakes in Vrancea.

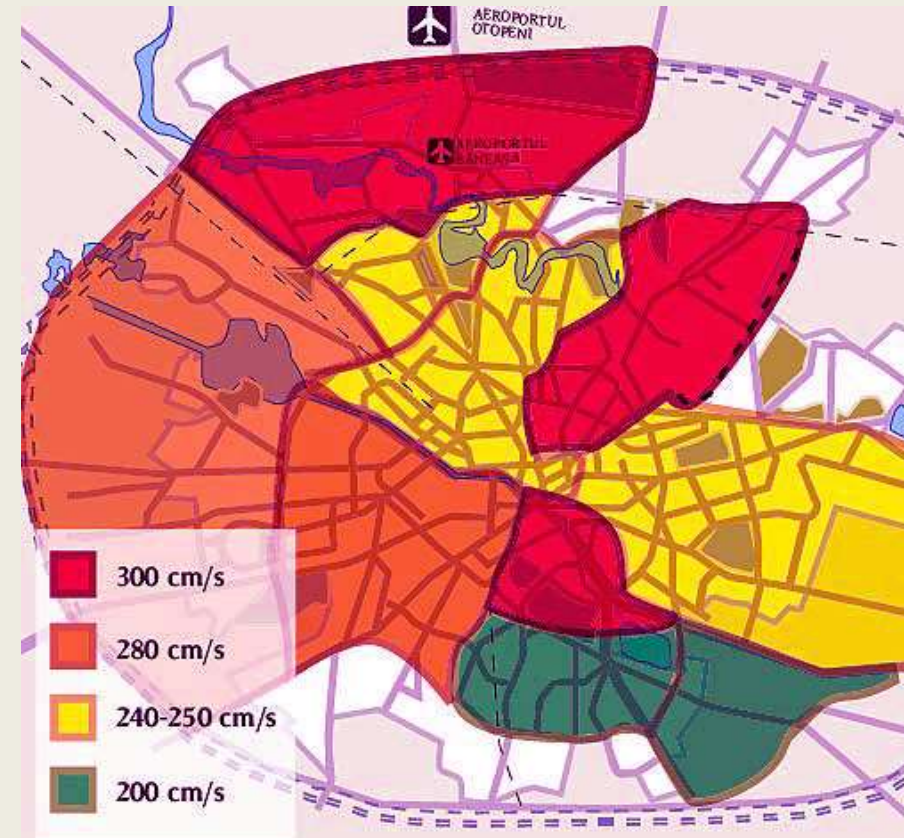
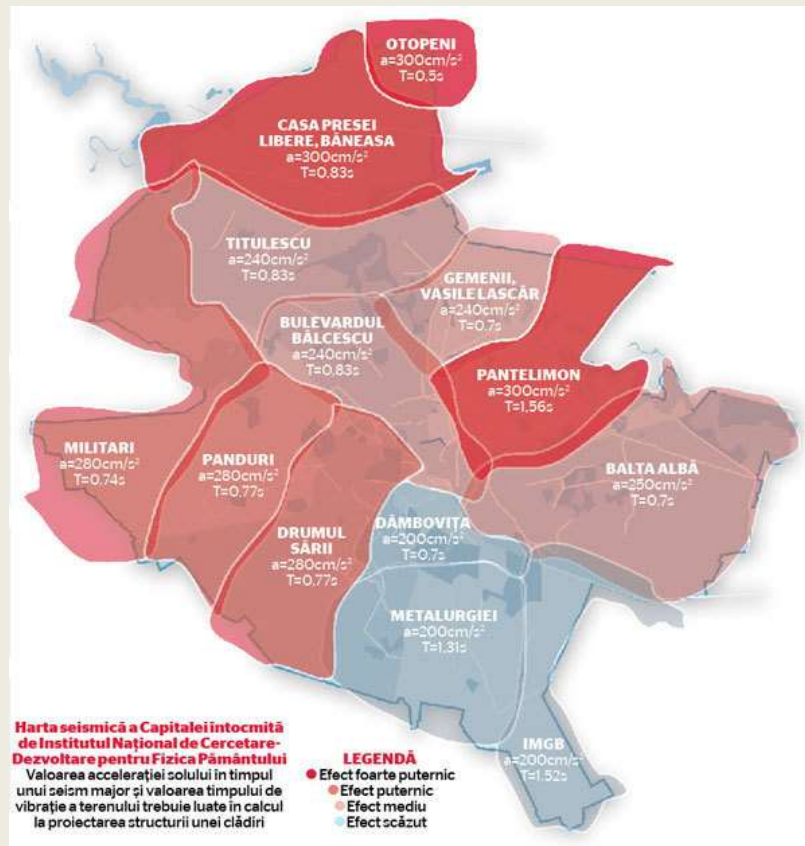
Earthquakes in areas such as Banat, Crisana, Maramures, Fagaras or Dobrogea (the list can be extended) have local effects, limited in extent.





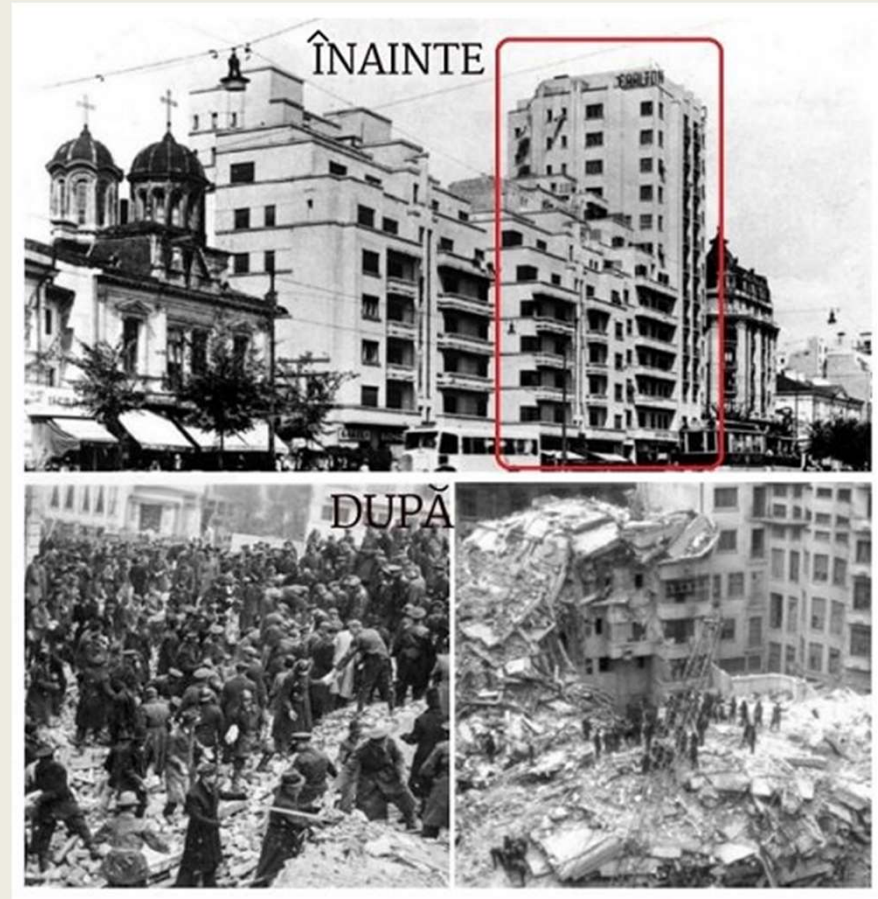
# SEISMIC CONDITIONS

## SEISMIC ACTION – Bucharest zoning maps



# SEISMIC CONDITIONS

## SEISMIC ACTION – Bucharest earthquakes



3.39 a.m., November, 10, 1940 – 7.4 Richter scale; 133 km depth; 45 s duration  
Carlton Building (47 m height) collapsed





# SEISMIC CONDITIONS

## SEISMIC ACTION – Bucharest earthquakes



9.22 p.m., March, 4, 1977 – 7.4 Richter scale; 147 km depth; 56 s duration  
32 buildings collapse; more than 1570 victims

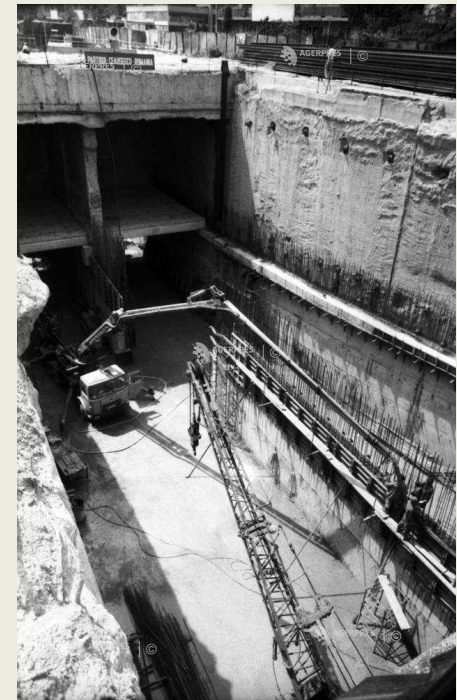
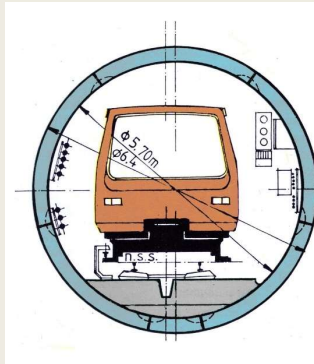




# BUCHAREST METRO HISTORY



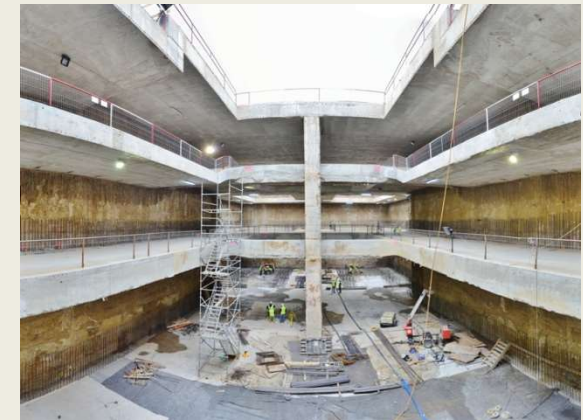
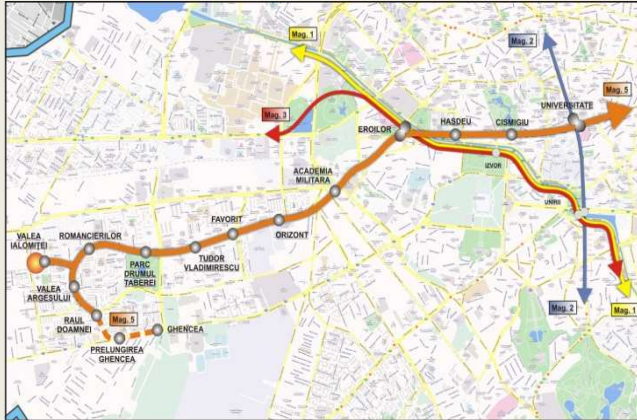
February, 1975 – start of designing  
 March, 1977 – start of construction  
 November, 16, 1979 – first section of M1 put into operation (Semanatoarea – Timpuri Noi)





# BUCHAREST METRO HISTORY

## NEW METRO LINE 5 – 2012-2020



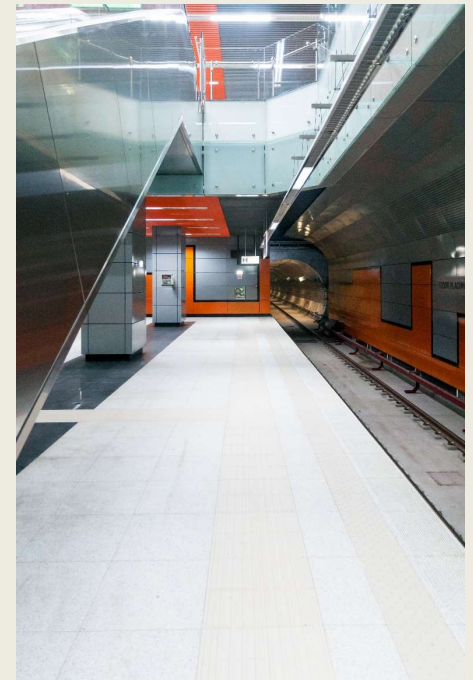
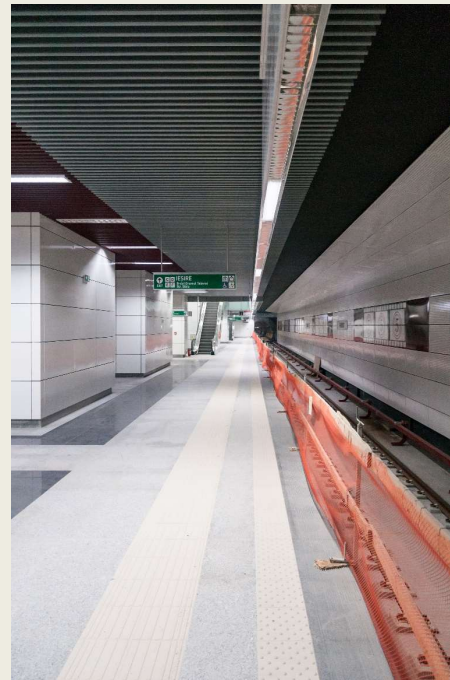
**Length: 9.29 km; Stations: 13; Stations length: 2.52 km; Depots: 1.**  
**TBM Tunnels length: 11.55 km; Cut-and-Cover Tunnels length: 0.81 km.**





# BUCHAREST METRO HISTORY

## NEW METRO LINE 5 – 2012-2020



Ime i prezime predavača

HKIG – Opatija 2021.



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**THANK YOU FOR YOUR ATTENTION !**

